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# Studies on Vibration-Based Damage Detection Methodologies for Structural Health Monitoring

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**ABSTRACT:** Early detection of damage is important for upgrading, assessment and maintenance of our ageing infrastructure like bridges. A recent development in the health monitoring of structures is Vibration-Based Damage Detection (VBDD). It has been represented as the process to typically monitor changes both in modal frequency and mode shapes. This paper presents the studies carried out on various modal parameters that are influenced by damage. Finite element studies were carried out on a truss model for extracting the modal parameters. In this study the truss structure was subjected to three damage cases by reducing the cross sectional area of a single element. Modal Analysis was performed and the frequency and mode shapes were extracted for both the healthy and unhealthy states of the truss structure. Using these parameters, the other parameters namely Frequency change, Damage Location Assurance Criterion (DLAC), Mode shape Curvature and Curvature Damage Factor (CDF) were studied. These parameter changes are useful to predict damage information, such as the presence and location of damage in a structure.

KEYWORDS: Vibration-based damage detection, Truss structure, Modal analysis, Damage detection

# I. INTRODUCTION

Structural health monitoring (SHM) is a new safety and management tool that aims to provide quantitative and reliable data on the real conditions of the structure, observe its evolution and detect the appearance of degradations. The primary objective of Structural Health Monitoring (SHM) is to determine if a structure is performing as expected or it has anomaly in its behavior as compared to the normal condition.

Vibration Based Damage Detection (VBDD) applied to structural health monitoring is very useful in interpreting the global vibration response of a structure to identify local changes (e.g. damages) in it. VBDDdepends on the change of dynamic characteristics of the structures. These characteristics arenatural frequencies, mode shapes and damping properties. These directlydepends on material properties, geometry and support condition which contributes to thestiffness and also the distribution of mass. Damage can cause change to any of these dynamiccharacteristics. Therefore, the VBDD method uses any change to dynamic or modalparameters of structures to identify, locate and detect the severity of the damage.T. Siebel et al (2011) [1] analyzed the application of vibration-based damage detection methods to a wind turbine in this paper.The target was to develop a system that detects and locates damage on a structure subjected to wind excitation.

The present work is to study the various modal parameters by requiring measurements of the changes in a few of the structure's natural frequencies and mode shapes between the undamaged and damaged states. It is observed to provide good predictions of both the detection of damage.



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#### **II. NUMERICAL MODELLING AND ANALYSIS**

Analytical evaluations were performed with the commercial, general-purpose finite element code ABAQUS, which can effectively depict the nonlinear behavior of structures. A specific objective of the analytical evaluation included the development of a finite element model that could correctly represent global bridge behavior and accurately predictdamage.

A two-dimensional truss model was developed to determine the overall structural response of the structure. The geometrical and material properties of the model are listed below.

٠	Span	-	6m
•	Height	-	1m
•	Young's modulus (E)	-	$2 \times 10^{11}  \text{N/m}^2$
٠	Density (p)	-	7850kg/m <sup>3</sup>
٠	Poisson ratio (µ)	-	0.3

The analysis was carried out by considering both the undamaged and damaged cases. Modal analysis was performed to extract the first five frequencies and mode shapes. The model of a 2D Howe truss structure which consists of 13 elements is shown in figure 1.



Figure 1: Finite Element Model Of Truss

Initially the modal parameters were extracted for the undamaged state in which the angle section is considered as  $100 \text{mm} \times 100 \text{mm} \times 12 \text{mm} (2259 \text{mm}^2)$ . Then damage is induced by reducing the cross-sectional area of a single element (element 11) of the structure. Three damage cases are specifically studied where the cross sectional area reduction is achieved by changing the angle section considered. All damage types considered are summarized in table 1 and the mode shapes for damage case 1 is shown in figure 2.



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#### Table 1: Considered Damage Cases

Damage Cases	Cross- Sectional Area (mm <sup>2</sup> )	Angle section (mm)
1	1402	$75 \times 75 \times 10$
2	1002	45×45×10
3	2250	$25 \times 25 \times 5$



Figure 2: First Five Mode Shapes Of Damage Case 1

# **III. STUDY ON MODAL PARAMETERS**

Structural damage often causes a loss of stiffness in one or more members of a structure that affects its modal frequencies and mode shapes. Based on these changesmany methods have been developed to detect the location and size of damage. In this study, damage was predicted noting the change in natural frequencies and mode shapes. For localization of damage other dynamic parameters such as Damage Location Assurance Criterion, Curvature of the mode shape and Curvature Damage Factorhave also been studied. The results of this study were generated using MATLAB which are discussed below.



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### A.FREQUENCY CHANGES

The use of natural frequency as a diagnostic parameter in structural assessment procedures using vibration monitoring is based on the fact that natural frequencies are sensitive indicators of structural integrity. Thus, an analysis of periodical frequency measurements can be used to monitor structural condition. The presence of damage or deterioration in a structure causes changes in the natural frequencies of the structure. The most useful damage location methods are probably those using changes in frequencies because frequency measurements can be quickly conducted and are often reliable [2]. The frequency values obtained from numerical analysis are tabulated in table 2.

Table 2: Natural Frequencies Of The Truss Structure

Mode number	Undamaged state	Damage case 1	Damage case 2	Damage case 3
1	32.841	32.752	32.730	25.643
2	78.880	74.634	70.647	65.269
3	142.62	136.77	129.64	125.59
4	178.31	178.26	178.11	170.68
5	340.06	320.97	320.97	314.28



Figure 3: Changes In Natural Frequency

Figure 3 summarizes the natural frequency obtained during each undamaged and damaged cases. No change in the dynamicproperties can be observed until the final level of damage is introduced. At the final level of damage the natural frequencies for the all the modes have decreased. This indicates the presence of damage in the structure.

# B. DAMAGE LOCATION ASSURANCE CRITERION (DLAC)

The Damage Location Assurance Criterion can be defined using a correlation approach.DLAC approach identifies damage by evaluating the linear correlation between frequency change vectors obtained by numerical measurements.



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The final estimation for the linear correlation between frequency change vectors is accomplished by the DLAC equation which is restricted to positive values between 0 and 1. DLAC values close to unity indicate a strong correlation and therefore a potential damage location.

$$DLAC = \left| \frac{\left( \Delta F^T \Delta f \right)^2}{\left( \Delta F^T \Delta F \right) \left( \Delta f^T \Delta f \right)} \right|$$

Where as,  $\Delta F$  = undamaged state frequency

 $\Delta f$  = damaged state frequency



DLAC values lies in the range of 0 to 1 with 0 indicating no correlation and 1 indicating an exact match between the patterns of frequency changes[3].

DLAC value for 3 cases are,

- Case 1: 0.99
- Case 2: 0.99
- Case 3: 1.00

This indicates that the DLAC value for the three damage cases were nearing 1, which assures the damage in the element no.11. The DLAC value for case 1 and 2 approximately specify the damage. But, the DLAC value for the case 3 accurately signifies the damage. It was found that more accurate damage identification can be obtained, if the frequency of the damaged state decreases with respect to the undamaged state frequency.

### C. MODE SHAPE CURVATURE

This method is based on direct consequence of damage, as reduction of flexural stiffness of a structure in correspondence with the damaged regions. This increases the amplitude of curvature at damaged region and so can be used to detect and locate damage.

The curvature mode shapes are derived using a central difference approximation as follows

### $K_i = (W_{i+1} + W_{i-1} - 2W_i) / h^2$

where w is the modal displacement at i<sup>th</sup> point, h is the length of the specimen.[4]



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Figure 5: Mode Shape Curvature For Three Modes

From figure 5 it is observed that the maximum curvature mode shape is highest in case 3 subjected to heavier damage.Thus results showed that the study of displacement mode shape can be a good indicatorof damage location. D. CURVATURE DAMAGE FACTOR (CDF)

Curvature Damage factor can be derived as the differences in modal curvature averaged over all modes which give a clearidentification of damage locations. Hence the method can be adopted with confidence for accurate modal parameter evaluation and subsequent application for damage identification. To summarize the results for all the modes, the curvature damage factor as proposed by Wahab and Roeck is calculated, which can be written as [5]

$$CDF = \frac{1}{N} \sum_{i=1}^{N} |u_{und} - u_d|$$
  
Where,

Ν = the total number of modes to be considered,

- = the curvature mode shape of the intact structure u<sub>und</sub> u<sub>d</sub>
  - = curvature mode shape of the damaged structure



Figure 6: Cdf

The CDF value is highest at the location of severe damage case 3. Using this parameter the location of crack can be precisely identified.



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#### **IV. SUMMARY AND CONCLUSIONS**

The intention of this paper is to detect damage by extracting the modal parameters of structures. Numerical studies were carried out on a truss structure using ABAQUS software for undamaged and damaged states. Frequency and mode shapes were extracted as a result of modal analysis. Using these parameters the various damage indicating methods were studied. The results of this study were generated using MATLAB.

Basedon the results of various methods, it can be concluded as,

- A gradual decrease in frequency occurs correspondingly as the cross sectional area decreases.
- Natural frequency changes alone may not be sufficient for a unique identification of the location of structural damage
- A new correlation coefficient termed the Damage Location Assurance Criterion has been studied to provide reliable information about the detection and location of damage.
- The maximum mode shape curvature is highest in region of severe damage.
- A better localization of damage could be done by curvature damage factor which shows more sensitivity than the mode shape curvature.

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