

SCIENCE | ENGINEERING | TECHNOLOGY

e-ISSN: 2319-8753 | p-ISSN: 2347-6710

EDIA

# INTERNATIONAL JOURNAL OF INNOVATIVE RESEARCH

IN SCIENCE | ENGINEERING | TECHNOLOGY

Volume 10, Issue 8, August 2021

INTERNATIONAL STANDARD SERIAL NUMBER INDIA

### Impact Factor: 7.569

9940 572 462

6381 907 438

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International Journal of Innovative Research in Science, Engineering and Technology (IJIRSET)



| e-ISSN: 2319-8753, p-ISSN: 2347-6710| <u>www.ijirset.com</u> | Impact Factor: 7.569|

|| Volume 10, Issue 8, August 2021 ||

| DOI:10.15680/LJIRSET.2021.1008068 |

## Soil Stabilization with Fly Ash & Rice Husk

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**ABSTRACT**: The objective of this paper is to upgrade expansive soil as a construction material using rice husk ash (RHA) and fly ash, which are waste materials. Remolded expansive clay was blended with RHA and flyash

and strength tests were conducted. The potential of RHA-flyash blend as a swell reduction layer between the footing of a foundation and sub grade was studied. In order to examine the importance of the study, a cost comparison was made for the preparation of the sub-base of a highway project with and without the admixture stabilizations. Stress strain behavior of unconfined compressive strength showed that failure stress and strains increased by 106% and 50% respectively when the flyash content was increased from 0 to 25%. When the RHA content was increased from 0 to 12%, Unconfined Compressive Stress increased by 97% while CBR improved by 47%.

KEYWORDS: Soil Stabilization, Fly Ash, Rice Husk

#### I. INTRODUCTION

Clays exhibit generally undesirable engineering properties. They tend to have low shear strengths and to lose shear strength further upon wetting or other physical disturbances1. They can be plastic and Compressible and they expand when wetted and shrink when dried. Some types expand and shrink greatly upon wetting and drying – a very undesirable feature. Cohesive soils can creep over time under constant load, especially when the shear stress is approaching its shear strength, making them prone to sliding. They Develop large lateral pressures. They tend to have low resilient modulus values2. For these reasons, clays are generally poor materials for foundations3. The annual cost of damage done to non-military engineering Structures constructed on expansive soils is estimated at \$220 million in the United Kingdom and many billions of dollars worldwide4.

Flyash5-7 was successfully used for stabilizing expansive clays. The strength characteristics of flyash stabilized clays are measured by means of unconfined compressive strength (UCS) or California Bearing Ratio (CBR) values. Depending upon the soil type, the effective flyash content for improving the Engineering properties of the soil varies between 15 to 30% 8-10. Rice Husk Ash (RHA) is obtained from the burning of rice husk.

#### II. METHODOLOGY

Fly ash is residual material remained after combustion of coal in thermal power plant. Fly ash contains fine particles of silicon dioxide (Sio<sub>2</sub>), aluminium oxide, iron oxide and calcium oxide. Fly ash has been used in many civil engineering projects successfully. Fly ash provides stability to sub grade, reduce earth pressure and also improves stability of slopes. Usually fly ash is mixed with clayey soils to improve properties as these soils cannot be used directly for construction due to their unfavorable properties.RHA is a carbon neutral green product. Lots of ways are being thought of for disposing them by making commercial use of this RHA. RHA is a good super- pozzolan.

- 1. Natural soil.
- 2. 92 % Soil + 8% Fly ash
- 3. 86 % Soil + 14% Fly ash
- 4. 82 % Soil + 18% Fly ash
- 5. 76 % Soil + 24 % Fly ash

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#### 2.1 INDEX PROPERTIES OF SOIL Plasticity Index

Plasticity index of soil was 5 % which is numerical difference between liquid limit and plastic limit.

#### 2.2 Type of soil

The type of soil was checked from plasticity chart. It was seen that soil which is taken for this study is intermediate plasticity. As per IS 1498 soil is classified as CI.

Engineering properties are indicated by index properties of in table 2.1.

SR.NO	PROPERTIES	VALUE	CONFIRMINGTOIS
			CODE
1.	Specific gravity(G)	2.60	IS 2720 : Part 3 : Sec
			1 :1980
2.	Maximum dry density(MDD)	2.124 gm/cc	IS 2720 : Part VII: 1980
3.	Optimummoisture content(OMC)	8.4 %	IS 2720 : Part VII: 1980
4.	Natural moisture content	8.6 %	IS 2720 : Part 2 : 1973
5.	Liquid limit	24	IS 2720 : Part 5 : 1985
6.	Plastic limit	19	IS 2720 :Part 5 : 1985

#### Table2.1 Geotechnical Properties of soil

#### 2.3 Compaction

The tests were performed in accordance with ASTM D 1557. The specimens were of 102mm diameter and 116mmheight. The degree of compaction of soil influences several of its engineering properties such as CBR value, compressibility, stiffness, compressive strength, permeability, shrink, and swell potential. It is, therefore, important to achieve the desired degree of relative compaction necessary to meet the required soil characteristics.

#### **2.4UCS**

The UCS tests were performed in accordance with ASTM D 2166. The sample sizes were of 40mm diameter and 80mm length. At the Optimum Moisture Content (OMC) and maximum dry unit weight values of the n atural soil, the t ests were performed.

#### 2.5 CBR

The CBR tests were conducted in accordance with ASTM D 1883. The sample sizes were of 152mm diameter and 126mm length. At the OMC and maximum dry unit weight values of the natural soil, the tests were performed.

#### 2.6 Swelling

Consolidation test (ASTM D 2435) setup was used for determining the cyclic swell-shrink behavior of the soil. The sample sizes were 76mm and 50mm in diameter and height respectively. The samples were prepared at Proctor's dry densities. The RHA was mixed with 15% flyash and compacted to dry unit weight of 5.5kN/m3 at a moulding water content of 120%. The compacted admixture was cured for 14 days and placed over the expansive soil. The efficacy of RHA as a cushioning layer between the foundation and subgrade was also tested using the consolidation test.

#### **III. CONCLUSION**

1 Stress strain behaviour of unconfined compressive strength showed that failure stress and strains

increased by 106% and 50% respectively when the fly ash content was increased from 0 to 25%.

2. When the RHA content was increased from 0 to 12%, CBR improved by 47%.

3. The optimum RHA content was found at 12% for both UCS and CBR tests.

4. The vertical movement of clay soils with cushioning material stabilizes after 3 cycles of swelling

5. When the RHA content was increased from 0 to 12%, CBR improved by 47%.

6. The optimum RHA content was found at 12% for both UCS and CBR tests.

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Impact Factor: 7.569





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