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Experimental Study on Impact Resistance of Steel Fibre Reinforced Concrete

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ABSTRACT: This study describes the impact resistance of fibre reinforced concrete with inclusion of steel fibres .The steel fibres are added in the range of 0.5%-2.5% of volume of mix. A simple economical and practical drop weight impact testing machine is used to determine the impact resistance for steel fibre reinforced concrete [SFRC]. Impact test are carried out on SFRC with aspect ratio (l/d) as 60. The specimens were tested at 7 days,14 days and 28 day strength for concrete grade M25 and workability test. The test result shows the variation of impact energy strength with different volume fraction of fibres. Steel fibres can be defined as discrete, short length of steel having ratio of its length to diameter (i.e. aspect ratio) in the range of 20 to 100 with any of the several cross-sections, and that are sufficiently small to be easily and randomly dispersed in fresh concrete mix using conventional mixing procedure. The random distribution results in a loss of efficiency as compared to control cracking. In many situations it is prudent to combine fibre reinforcement with conventional steel reinforcement to improve performance. This paper examines the impact responses of fibre reinforced concrete and to evaluate the resistance levels of FRC beams when subjected to impact loadings.

KEYWORDS: Impact resistance, steel fibre, toughness, tensile properties, aspect ratio.

I. INTRODUCTION

Steel fibre (SF) is the most popular type of fibre used as concrete reinforcement. Initially, SFs are used to prevent/control plastic and drying shrinkage in concrete. Further research and development revealed that addition of SFs in concrete significantly increases its flexural toughness; the energy absorption capacity, ductile behaviour prior to the ultimate failure, reduced cracking, and improved durability. This paper reviews the effects of addition of SFs in concrete, and investigates the mechanical properties, and applications of SF reinforced concrete (SFRC).

The beneficial influence of SFs in concrete depends on many factors such as type, shape, length, cross section, strength, fibrecontent; SFs bond strength, matrix strength, mix design, and mixing of concrete. The addition of SFs in the conventional reinforced concrete (RC) members has several advantages such as 1- SFs increase the tensile strength of the matrix, thereby improving the flexural strength of the concrete. 2- The crack bridging mechanism of SFs and their tendency to redistribute stresses evenly throughout the matrix contribute to the post-cracking strength and restraining of the cracks in the concrete. 3- Increase ductility of the concrete. 4- SFRC is more durable and serviceable than conventional RC.The main objective of this project is to study the Impact resistance parameter offibre reinforced concrete with mix proportion of fibres for M25 grade concrete and comparing with the conventional concrete and to know the optimum percentage of addition of fibres to concrete and finding maximum ratios.

II. EXPERIMENTAL STUDY

The experimental investigation was focused on the effect of various fibredosages on impact resistance of FRC. Mix proportion was designed using IS 10262-2009 and IS 456-2000 with mean target strength of 39.36 MPa (M25) for control mix. Portland Pozollona cement of grade 53 was used in this study. A coarse aggregate with a maximum nominal size of 20 mm and a fine aggregate with a fineness modulus of 3.38 were used in the experiment. Hooked-end



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steel fibres wereused; their geometry and apparent shape are shown in Fig.1and their properties are listed in Table 1. Super plasticizer of SP-430 was used to adjust the workability of mixtures.



DESCRIPTION
0.3mm-0.7 mm
25-35 mm
7900 Kg/m ³
2.1×10^{5}
GOOD
POOR
GOOD
5-35%

Table 1: Properties of steel fibre

Figure 1: Steel Fibres

III. MIX DESIGN

In this study, water cement ratio of 0.47 was adopted for M25 grade concrete and steel of 0.5%, 1%, 1.5% volume fractions were used.

Constituents	Content kg/m ³ of concrete
Cement	336
Fine aggregate	834
Coarse aggregate	1206
Water content	158
Fibres	
0.5%	12.67
1%	25.34
1.5%	38.01
Super plasticizer	1.34 kg/m^3 (0.4% by the weight of cement)

Table 2: Contents of mix proportion

IV. TEST METHOD

The impact test was performed in accordance with the impact testingprocedures recommended by ACI Committee 544. The test was carried out by dropping a hammer weighing 44.7N (10 lb) from a height of 457mm (18 inch) repeatedly on a 64mm(2.5inch) hardened steel ball, which is placed on the top of the centre of the cylindrical specimen (disc). The test continued until it fails. For each specimen, two values were identified corresponding to initial and ultimate failure. The former value measures the number of blows required to initiate a visible crack, whereas the latter measures the number of blows required to initiate and propagate cracks until ultimate failure. According to the ACI committee, the ultimate failure occurs when sufficient impact energy has been supplied to spread the cracks enough so that the test specimen touches the steel lugs. However, in this study, if the specimen separated completely into halves before touching the lugs, then this was declared the point of ultimate failure. From this test, impact energy absorption capacity of each specimen was calculated by the equationImpact Energy (E imp) = $(0.5mv^2)$ Where m =mass of the hammern= no of blowsV= Impact velocity.

V. RESULTS AND COMPARISON

The results are based upon the number of blows that are required to initiate initial crack and number of blows that are required for final crack. Impact resistance of mono fibre reinforced concrete was calculated at different curing periods and compared to conventionalconcrete. The impact energy wascalculated as follows

 $H=(gt^2)/2$

V= g*t

 $m{=}\;W{/}g$



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where V is the velocity of hammer by impact g is acceleration due to gravity, t is the time required for hammer to fall from a height of 457mm, H is the height of the hammer, weight of the hammer substituting the known values we get

457= (9810 t²) /2 t=0.3052 sec V= 9810 x 0.3052 = 2994.01 mm/s The above value substitute in Impact energy equation EImp= (0.5 x 44.3 x 2994.012)/9810= 20.345 KN-mm

Table 3: Impact resistance for initial crack							
Fibre	Number	Impact	% increase in				
dose	of blows	energy	impact energy				
	for initial	(KN-					
	crack	mm)					
PCC	18	441.59	-				
0.5%	111	2455.75	450.00				
1%	158	3405.96	663.64				
1.5%	231	4903.15	995.45				

Table 4: Impact resistance for final crack

Fibre	Number	Impact	% increase in	
dose	of blows	energy	impact	
	for initial	(KN-	energy	
	crack	mm)		
PCC	25	601.33	-	
0.5%	148	3151.41	411.67	
1%	188	4007.97	536.67	
1.5%	301	6286.61	930.00	



Figure 2: Number of blows taken for initial crack

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Figure 4: comparison of number of blows of crack



Figure 5: Percentage increase in impact energy

Table 5: Comparison between compressive strength and number of blows

	Compressive strength	Initial crack	Final crack
PPC	34.56	18	25
0.5% of fibre	35.68	111	148
1% of fibre	38.26	158	188
1.5% of fibre	39.36	231	301



Figure 5: Percentage increase in impact energy

Increase in fibre dosage resulted inincrease of number blows required for initial and final crack. Maximum number of blows wasfound to be 231 and 301 for initial and final cracks respectively fora fibre dosage of 1.50%. The difference between number blows for final crack and initial crack increased with increase in fibre dosage. Maximum difference was seen for the fibre dosage of 1.50%. Impact energy was directly associated with the number of blows. Impact

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energy increased with increase in fibre dosage for both initial crack and final crack. More increase in impact energy was observed for initial crackas compared to final crack for all the fibre dosages. Compressive strength also followed the same trend as that of impact resistance. Although the change in compressive strength was not as substantial as that of impact resistance, slight increase in compressive strength was observed with increase in fibre dosage.

VI. CONCLUSION

In this context, the composite was fabricated with steel fibre. Their mechanical properties like impact strength from impact test and compressive strength are investigated and the following conclusions were drawn.

- Steel fibres were found to be effective in improving impact resistance of the concrete.
- Impact resistance increased with increase in fibre dosage and the maximum resistance in terms of number of blows was found for a fibre dosage of 1.50%
- From the above experimental data, it can be concluded that because of its mechanical properties Steel fibrereinforced composites can be extensively used in highway and airport pavements, hydraulic structures, etc.

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