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### Design and Analysis of Circular Water Tank with Rigid Base Resting on Ground

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ABSTRACT: In some locations, A water tank is a structure that stores water for the purpose of providing drinking water to households, cooling water for industries, and irrigational water for agricultural use. Water tanks are categorized based on their shape and structural location. Water tanks come in a variety of capacities, depending on the demands of a municipal organization. Water covers approximately 71% of the earth's surface. Dead and live loads are applied using IS codes. A water storage container is a ground-level water tank. It was discovered that limit state design had a reduced cost. The cost of a circular water tank is less than that of a square tank. A circular water tank with a circular wall designed as a liquid retention structure demonstrates the theory behind the design of this work. To resist lateral stresses, the water tank is modelled and studied using dynamic analysis, and the design is done manually using the working stress method. Staad pro software is used for the design. This study provides numerous design requirements that influence the structure's strength and support condition for safe tank construction. The focus of this case study is on liquid retaining structures that are composed of reinforced concrete and sit on the ground.

KEYWORDS: Water, Circular Tank., STAAD Pro, analysis, Design as per IS code: IS-3370:2009, IS-456:2000.

#### I. INTRODUCTION

A water tank is a field used to store water. Water is regarded as the source of life for all creatures, as well as an essential component of a healthy lifestyle. Safe drinking water is one of the most important factors in maintaining a healthy lifestyle. As one cannot live without water, the demand for safe and easy water is increasing by the day. Water tanks are used to store water for a variety of applications such as drinking water, irrigation agriculture, fire suppression, agricultural farming (both for plant life and livestock), chemical manufacturing, food training, and many others. The overall design of the tank, as well as the desire to manufacture substances and linings, are all water tank criteria. As a result, water must be stored simply and effectively. In garage reservoirs and water tanks, liquid petroleum, petroleum products, and analogous liquids are routinely stored. The force evaluation of the reservoir or tank is the same regardless of the chemical type of the product. To prevent leakage, all tanks are designed as crack-free systems. Water or raw petroleum preserving slabs and partitions might be reinforced concrete with a suitable reinforcing cowl. No extra treatment is required because water and petroleum react with concrete. The need for a water tank is as old as civilization itself, as it provides a storage space for water that may be used for a variety of purposes. Water tanks are commonly classified as round, square, or conical, depending on their size and position. The tanks could be made of metal or concrete. Tanks that sit on the floor are usually round or square and are used to hold large amounts of water. The overall layout of the tank, as well as the desire to create substances and linings, are all considered water tank factors. Water tank layout components must be followed as closely as possible to books, and masses must be properly carried out.

#### II. METHODOLOGY

#### **Manual Design**

Design a circular water tank for a capacity of 600000 liters resting on the ground. It is fixed @ base and free at the top. Take $\gamma_w = 10$ kn/m<sup>3. Use</sup> M30 / Fe415 materials.

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Provide 5m depth let the free board be 0.3m Let D=diameter of the tank Considering 20mm thick waterproof plaster  $\therefore$  Net diameter= D-(2x0.02) = (D-0.04) mNet height= (5-0.3-0.02) =4.68m Volume=600000 litre = $600m^3$  $\therefore 600 = \frac{\pi}{4} x (D - 0.04)^2 x 4.68$ D=12.81 Provide D=12.85, H=5m Assume thickness t=250mm=0.25m  $\therefore \frac{H^2}{Dt} = \frac{5^2}{12.85X0.25} = 7.78$ Hoop tension consideration From table 9, pg.no 35 of IS:3370 (part iv) In the table, we have tosearch for the max coefficient Here max coefficient is at 0.6H Hoop tension= $k_1 \frac{\gamma HD}{2} \gamma$  = unit weight of water  $T=k_1\frac{10X5X12.85}{2}$ 2  $=321.25k_{1}$  $@0.6H => H^2/Dt$ 6 - 0.514 7.78 -? 8.0 - 0.575 By interpolation k<sub>1</sub>=0.5684 T=321.25X0.5684=182.60KN :.Hoop steel =  $\frac{182.60X10^3}{2}$  = 1404.61 mm<sup>2</sup> t=250mm  $Ast_{min} = \frac{0.24}{100} \times 1000 \times 250$  $=600 \text{mm}^2$ Max spacing =300mm  $\frac{\pi}{4}X_{12}X_{100}$ ∴Spacing of 12mm bars=4 =161.03mm 1404.61 Provide 12mm bar@160mm c/c (both faces)  $\therefore \text{Ast provided} = \frac{2X\frac{\pi}{4}X12^2X1000}{-}$  $\frac{x_{12}^2 x_{1000}}{160} = 1413.72 \text{mm}^2$ Tensile stress in concrete= $\frac{1}{bXt+(m-1)Astp}$  $182.6X10^3$  $= \frac{1}{(1000X250) + (9.33-1)X1413.72}$  $=0.7 \text{ N/mm}^2 < 1.5 \text{ N/mm}^2$ HENCE OK

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#### **BENDING MOMENT**

 $H = k_2 \gamma_w H^3$  $=k_2 X 10 X 5^3$  $=1250k_{2}$ From table 10 of IS:3370 (part iv) In the table, we have tosearch for max coefficient Here max coefficient is @1.0H 6= -0.0187 7.78= ? 8=-0.0146 By interpolation, k<sub>2</sub>-0.015.47 M= (1250x0.015047) =-18.81KNm D=250-45-10-5=190mm From table 10, Pg.no 36 of IS:3370 (part iv) M=Qbd<sup>2</sup>  $Q=0.5\sigma_{cbc}kj$  $\sigma_{cbc}=10MPa$ mσcbc k=-=0.4178=0.5x100x0.4178 j=1-k/3 = 0.86Q=1.79  $18.81 \times 10^{6} = 1.75 \times 1000 \times d_{reg}^{2}$ dreq = 102.51mm <d availableAst required =  $\frac{m}{\sigma st j d}$  =  $\frac{18.81 \times 10^6}{130 \times 0.86 \times 190}$  = 885.509 mm<sup>2</sup>

Provide 10mm bar @ 170mm c/c [462mm<sup>2</sup>] for full height of the wall + 10mm bar @170mm c/c 462mm<sup>2</sup> for short length of 1000mm Ast=462+462=924mm<sup>2</sup>

#### Max shear @ base

From table 2 of IS:3370 (part iv) V=CoefxYw H<sup>2</sup> 6=+0.197 7.78 =? 8=+0.174 By interpolation, Coefficient =0.177 V=0.177 X 10 X 5<sup>2</sup> =44.25 KN

#### **Nominal Shear Stress**

 $\begin{aligned} \tau_{\rm v} &= \frac{44.25 \times 10^3}{1000 \times 190} = 0.2328 \text{ N/mm}^2 \\ \frac{100 As}{bd} &= \frac{100X924}{1000 \times 190} = 0.486\% \\ \tau_{\rm c} &= 0.3 \text{ N/mm}^2 \text{ for } P_t = 0.486\% \\ \text{Here } \tau_{\rm c} > \tau_{\rm v} \\ \text{HENCE OK} \end{aligned}$ 



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#### STADD PRO DESIGN

Step 1:Geometry design Put all nodal and connect beam



Fig 1: All plates are connected

#### Step 2: Design an end property



Fig 2: Isometric view of the nodes of tank

Fig 3: Top view of the nodes of tank

Step 3: Material



Fig 4:Assigning thickness of tank



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Step 4: Select all members and given property



Fig 5:Applied thickness of tank

Step 5: Load and Combination



Fig 6: Load and combination



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Step 6: Analysis and print

Add IS code and specification IS 456

Change Perform Direct Analysis Generate Floor Spectrum Nonlinear Analysis Perform Analysis PDeta Analysis Perform Cable Analysis Print Option No Print Load Data Statics Check Statics Load Mode Shapes Both All	Perform Im	perfection Analy	sis Perf	orm Buckling Analysi	is Perfor	m Pushover Analysis
Perform Analysis PDeta Analysis Perform Cable Analysis Print Option  Print Option  No Print  Load Data  Statics Check  Statics Load  Mode Shapes Both All	Change	Perform Direct	Analysis	Generate Floor Sp	ectrum	Nonlinear Analysis
Print Option No Print Load Data Statics Check Statics Load Mode Shapes Both All	Perfor	m Analysis	PD	elta Analysis	Perfor	n Cable Analysis
		Print C N L S S S S M B B O A	Dption lo Print oad Data tatics Che tatics Loa lode Shap oth	ck d		

Fig 7:Analysis and printing commands

#### **III. EXPERIMENTAL RESULTS**

#### STEP 7: RUN PROGRAM

2 anono zunajora una oceaga	15:18: 9	Results Setup		
Factor at equation= 1000 of = 1206	15:18: 9	Loads Range Result View Option	ne	
Factorization done, # of non zeros= 79101	15:18: 9	Hunge Headir Herr optio	113	
++ Advanced Solver Saving displacement.	15:18: 9	O D-faul Faultan		
++ Calculating Member Forces.	15:18: 9			~
++ Analysis Successfully Completed ++	15.10.0	C Envelope of Load Cases in Se	alacted List	
++ Processing Element Forces.	15:10: 9	Citvelope of Load Cases in Se	elected List	
++ Processing Element Corner Forces.	15:10: 9	Load Cases		
++ Creating Displacement File (DSP)	15.10.10			
++ Creating Reaction File (RFA)	15:18:10	Available:		Selected:
++ Creating Element Stress File (EST)	15:18:10			1.0
++ Creating Element JT Stress File (EJT)	15:18:10		>	TUL
++ Creating Element JT Force File (ECF)	15:18:10			211
++ Creating Design information File (DGN)	15:18:10		>>	3 COMBINATION LOAD CASE 3
++ Done.	15:18:10			
0 Error(s), 0 Warning(s), 0 Note(s)				
++ End STAAD.Pro Run Elapsed Time = 2 Secs			<	
nev	tank.anl			
			<<	
O View Output File				
O Go to Post Processing Mode			6	
O Stay in Modeling Mode	Done			OK Apply Help

Fig 8: The result of analysis of Staad pro





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STEP 8: PRINCIPAL STRESSES ON PLATES



Fig 10: Principal stresses of plates

STEP 9: RESULT



Fig 11: Elevation Detail by Manual Calculation

Fig 12: Elevation Detail by Staad Pro

#### IV. CONCLUSION

Water storage in the form of tanks for drinking and laundry, swimming pools for exercise and recreation, and waste matter alluviation tanks are becoming increasingly important in modern life. The design of a water tank is a time-consuming process. Manual design necessitates the use of numerous mathematical formulas and calculations. It takes time as well. As a result, the software provides a solution to the above mentioned challenge. There is a slight variation between software design values and manual calculation values.



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