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Seismic Performance Evaluation of Industrial Junction Tower

Shubham R Gurav¹, Akash C Arakere²

Post-Graduation Student, Department of Civil Engineering, BIET, Davanagere, Karnataka, India¹

Assistant Professor, Department of Civil Engineering, BIET, Davanagere, Karnataka, India²

ABSTRACT: The Industrial towers carry heavy loads at a sufficient and safe height from ground. In addition to their self-weight, they have to withstand all forces of nature like strong wind, earthquake and snow load. Therefore, Industrial towers should be designed considering both structural and electrical requirements for a safe and economical design. A model of the Industrial towers used effective element types on various components of Industrial towers for static and dynamic analysis. Further determine the static response and corresponding stress resultants of Industrial towers structure one static instant time on vertical and transversely position of Industrial towers using ETABS. Also studied free vibrational or modal analysis characteristics of the transmission tower by determine the frequencies and mode shapes of transmission tower using ETABS and validating the finite element-based results with closed form solution. At last elaborate study on the transient dynamic analysis of transmission tower using ETABS with emphasis on the evaluation of dynamic response of transmission tower due to time varying wind load with various wind velocity like displacement and axial force.

KEYWORDS:Seismic Evaluation, Industrial Tower, Junction Tower.

I. INTRODUCTION

Earthquakes have occurred on our planet since the beginning of time. During abuilding's existence, it may be subject to earthquakes, which are otherwise a very uncommonforce. The succession of vibrations is generated when lateral forces go through the ground. As a result of these oscillations, the earth and any buildings built upon it are subjected to dynamicloads or inertial forces that cause them to vibrate in a very intricate pattern. Structures mightbe damaged by these dynamic loads or inertial forces. Static (gravitational) and dynamic loads (such as wind load) are designed to be withstood elastically in the typical design method inplaces where seismicity is minor. However, if this design strategy is mandated in situation where seismic excitation has to be considered, this might lead to energy inefficient andeconomically unfeasible design choices. The result of adopting such a tactic is increased massand thus, seismic activity. Seismic design is based on the use of building methods, kinds, and criteria for constructing earthquake-prone buildings. The building might fall or be destroyeddue to its inadequate seismic architecture. Structures are constructed to withstand the types of earthquakes that are expected to occur at the site of construction in compliance with applicablebuilding rules to minimise the risk of collapse. Using seismic design, architects can ensure thatstructures are robust enough to withstand earthquakes without suffering damage. The ground, which is necessary before the structure type, the placement of the structure, and the application fseismic design and criteria, is the most fundamental necessity of seismic designs. The term"seismic resistant construction" describes the practise of using seismic design and buildingrules to create buildings that are resilient to earthquakes.

II. RELATED WORK

K.G.Vishwanath(2010) Publication in the International Journal of Civil and StructuralEngineering (2010) on "Seismic reaction of Steel braced Reinforced Concrete Frames." As perIS 1893:2002, a four-story structure was photographed in a seismic zone 4 area. Story drift isused as a performance metric for buildings. The investigation is then expanded to include atotal of eight and twelve stories. The most effective steel bracing is X, as determined by this study.

Seismic Analysis of High-Rise Steel Frame Building With and Without Bracing is the subjectof study by K.K.Sangle, K.M. Bajori, and V.Mhalungkar (2012). The purpose of this researchwas to evaluate the effectiveness of seismic analysis for high-rise steel buildings with and without bracing systems. The study's findings, based on a time-history



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analysis, indicate thatearthquake-induced structural behaviour may be significantly altered by the addition of bracingelements.

III. METHODOLOGY

Linear static analysis

Usually this is the result of the dynamic phenomenon of the structure that undergoes anearthquake. This dynamic analysis is not often used as it is complex and time-consumingmethod in daily design. Most structures undergoing seismic forces are designed with the ideathat the building is strained into the inelastic range.

Linear dynamic analysis

Static procedure is used only for short building, hence for tall structures having nonorthogonalor torsional irregularity the dynamic method of analysis is preferred. the seismicinput is modelled, but in both cases by using linear elastic analysis the corresponding splacement and internal forces are determined. As compared to that of linear static procedure of analysis high modes can be used in case of linear dynamic analysis procedure.

Response spectrum analysis

This method makes use of damping level and time history to measure pseudo spectralacceleration, displacement or velocity to provide dynamic behaviour of a structure. For eachstructural period experienced by the structure there is a smooth curve on that gives peakresponse in a response spectrum for the dynamic response this method is related with the type of the structure selection this method is useful for the decision making for the design of a structure. This method requires many building codes except for simple structure orcomplex structure.

Non-linear static analysis

This method which can also known as an analysis of the push over. It defines the capacitycurve. It includes the pattern of forces. These forces are applied to the structure comprising of properties which are nonlinear in nature. Then the total applied forces are plotted against areference displacement.

Non-linear dynamic analysis

This method is a combination of structure and ground motion details and so that this method affects the less uncertainty. In this way of analysis, some of the majority of the analysis requiressome codes for the generalization of the ground motion. To achieve structural response several different ground motion records are needed.

IV. EXPERIMENTAL RESULTS

1) DISPLACENEMT ALONG X AXIS

MODEL	X IN MM
ZONE 4	2.298
ZONE 5	3.446

In this Junction Tower Model in X direction in Displacement Loads Results for the model atZone 4 and Zone 5 as we can Clearly see the Displacement Increases with the increase of theZone also. Hence Zone 4 is the safer Zone for this Model for the consideration of theDisplacement along X Direction.

2) DISPLACENEMT ALONG Y AXIS

MODEL	Y IN MM
ZONE 4	3.321
ZONE 5	4.981

In this Junction Tower Model in Y direction in Displacement Loads Results for the model atZone 4 and Zone 5 as we can Clearly see the Displacement Increases with the increase of theZone also. Hence Zone 4 is the safer Zone for this Model for the consideration of theDisplacement along Y Direction.



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3) STOREY SHEAR ALONG X AXIS

MODEL	Y IN MM
ZONE 4	4.0898
ZONE 5	6.1347

In this Junction Tower Model in X $\overline{\text{direction}}$ in Story Shear Loads Results for the model atZone 4 and Zone 5 as we can Clearly see the Story Shear Increases with the increase of theZone also. Hence Zone 4 is the safer Zone for this Model for the consideration of the StoryShear along X Direction.

4) STOREY SHEAR ALONG Y AXIS

MODEL	Y IN MM
ZONE 4	5.3894
ZONE 5	5.3894

In this Junction Tower Model in Y direction in Story Shear Loads Results for the model atZone 4 and Zone 5 as we can Clearly see the Story Shear Increases with the increase of theZone also. Hence Zone 4 is the safer Zone for this Model for the consideration of the StoryShear along Y Direction.

5) AUTOLATERL ALONG X AXIS

MODEL	X IN KN
ZONE 4	6.7165
ZONE 5	10.0747

In this Junction Tower Model in X direction in Auto-lateral Loads Results for the model atZone 4 and Zone 5 as we can Clearly see the Story Shear Increases with the increase of theZone also. Hence Zone 4 is the safer Zone for this Model for the consideration of the Autolateralalong X Direction.

6) AUTOLATERL ALONG Y AXIS

MODEL	X IN KN
ZONE 4	6.7165
ZONE 5	10.0747

In this Junction Tower Model in Y direction in Auto-lateral Loads Results for the model atZone 4 and Zone 5 as we can Clearly see the Story Shear Increases with the increase of theZone also. Hence Zone 4 is the safer Zone for this Model for the consideration of the Autolateralalong Y Direction

V. CONCLUSION

- 1) In this Junction Tower Model in X & Y direction and considering the Results of thezone 4 for in X direction is found to be 2.298 mm and in Y direction it is found to be3.321mm. And the Graph Plotted Below Represents the Curve along X and Y axis with their Respective Colours which can be Easy to compare for further Analysis.
- 2) In this Junction Tower Model in X & Y direction and considering the Results of thezone 4 for in X direction is found to be 4.0898 mm and in Y direction it is found to be4.3115 mm. And the Graph Plotted Below Represents the Curve along X and Y axiswith their Respective Colours which can be Easy to compare for further Analysis.
- 3) In this Junction Tower Model in X & Y direction and considering the Results of thezone 4 for in X direction is found to be 6.7165 mm and in Y direction it is found to be6.7165 mm. And the Graph Plotted Below Represents the Curve along X and Y axiswith their Respective Colours which can be Easy to compare for further Analysis.
- 4) In this Junction Tower Model in X & Y direction and considering the Results of the Zone 5 for in X direction is found to be 3.446 mm and in Y direction it is found to be4.981 mm. And the Graph Plotted Below Represents the Curve along X and Y axis with their Respective Colours which can be Easy to compare for further Analysis.
- 5) In this Junction Tower Model in X & Y direction and considering the Results of thezone 5 for in X direction is found to be 6.1347 mm and in Y direction it is found to be6.4673 mm. And the Graph Plotted Below Represents the Curve along X and Y axiswith their Respective Colours which can be Easy to compare for further Analysis.
- 6) In this Junction Tower Model in X & Y direction and considering the Results of thezone 5 for in X direction is found to be 10.0747 KN and in Y direction it is found to be10.0747 KN. And the Graph Plotted Below Represents the Curve along X and Y axiswith their Respective Colours which can be Easy to compare for further Analysis.



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7) Hence we can Conclude that the this Industrial Tower Design model is capable towithstand in Zone 4 only. Seeing all the Results we can clearly see that as increase inzone 5 is effects the life span of Tower model. Hence the Tower model at Zone 4 isBetter to be Considered as per Results.

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