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Seismic Behaviour of Double Steel Plate Composite Wall Under Cyclic Loading. A Review

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ABSTRACT: For high strength, durability and resistance to external forces, steel and concrete composite wall are used in construction of high-rise building. This wall consists of concrete infill and steel plate connected to each other by using tie bars. In this paper, we studied the work done by various researchers on various behavior of double steel plate composite wall under cyclic loading with different types of boundary conditions, different shapes of wall, different shear stud and tie bar spacing etc.

KEYWORDS: Steel Plate, Cyclic Loading, Monotonic Loading, Composite Wall, Strength, Various Behavior.

I. INTRODUCTION

To resist the load, construction industries has been increases the use of steel plate composite wall. The various loads acting on the high rise building due to earthquake, like monotonic loading, cyclic loading and dynamic loading. Which is prevented by using double steel plate composite wall in the high rise building to make the structure more stable. In this paper our focus is cyclic behavior of double steel plate composite wall under seismic loading.

II. LITERATURE REVIEW

Hadi kenarangi, Emre Kizilarslan, Michel Bruneau[1] (2021) investigated the cyclic behavior of C-shaped composite plate shear wall concrete filled. It was a special lateral- force resisting system made of two steel plate as sandwiched panel which is filled with concrete infill called as a composite plate shear wall/ concrete filled (C-PSW/CF). Tie bar is used as connector between steel plate. Two large-scaled C-shaped concrete filled composite plate shear core wall subjected to flexural and axial loads together for testing with same dimensions but different axial loads were applied upto 19% of axial loading capacity. The specimens were designed to have aspect ratios of cross-section ($\gamma = b/h$), web ($\beta = c/b$), and flange ($\alpha = d/h$) within $\pm 35\%$ of those from the prototype model (i.e., defined by the practicing engineers on the Project Advisory Team to be $\gamma p = 0.33$, $\beta p = 0.21$, and $\alpha p = 0.07$). A572 Gr50 steel was used in construction of the flange and web plates e yield strengths of the plates were determined to be 54.6ksi (376.5 MPa) and 56.2ksi (387.5 MPa) for web and flange plates of the first specimen (which will be referred as Specimen C1 this point forward), and 52.65ksi (363 MPa) and 57.79ksi (398.4 MPa) for web and flange plates of the second specimen (which will be referred as Specimen C2 this point forward), respectively.

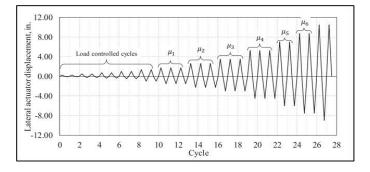
The walls were subjected to 19% and 15% axial loading before horizontal cyclic loads were applied as shown in figure 1 and test setup was shown in figure 2. similar buckling and fracture behaviors shown in both specimens. Ductility was more than 4 (4.02/-4.3 for positive and negative directions for both specimens) when flexural strength dropped to 80% of the peak value developed, and both specimens reached or exceeded their calculated plastic moment capacities in the positive and negative direction.

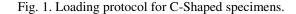
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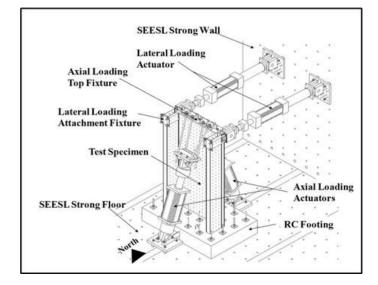


Fig. 2. 3D representation of test setup.

M. Nadir Olabi, Naci Caglar, M. Hasim Kisa, S. Bahadir Yuksel[2] (2021) conducted a numerical analysis on composite shear walls with steel sheets, used the L-shaped cold-formed steel sheets under lateral cyclic forces. Actual behavior of composite shear wall was reproduced by developing a macroscale numerical model using a fiber beam-column element with shear spring model. Three composite shear wall was experimentally tested and compared with open sees-based model and showed strong simulation ability. The mechanical properties of L-shaped steel sheets were studied by considering the parameter in terms of thickness and yield strength. Figure 3 shows the loading protocol for composite shear wall.

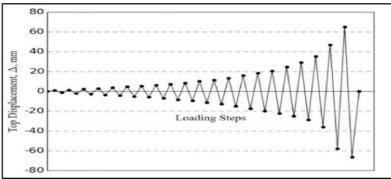


Fig. 3. Applied load protocol for CSW

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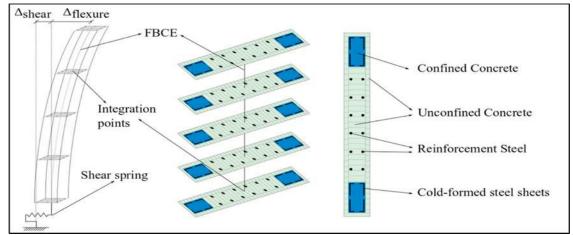


Fig. 4. Illustration of fiber beam-column element model

The mechanical properties of L-shaped steel sheets were studied by considering the parameters in terms of thickness and yield strength. Figure 4 illustrated the fiber beam column model. The lateral load carrying capacity was increased as the thickness and yield strength of steel plate increased.

Qiuhong Zhaoa, Yikang Lia, Ying Tianc[3] (2020) studied the cyclic behavior of double steel plate composite wall with flat and corrugated faceplate for mid to high-rise building made up of steel face plate connected with each other by tie bolts, concrete filled steel tubes and concrete.

| Specimen | Loading direction | Ko (kN/mm) | P_y (kN) | dy (mm) | Oy (%) | Pmax (kN) | d _{mate} (mm) | Omar (%) | d _u (mm) | θ _u (%) | μ |
|----------|-------------------|------------|------------|---------|--------|-----------|------------------------|----------|---------------------|--------------------|-----|
| CW-F1 | + | 115 | 515 | 9.4 | 0.63 | 1083 | 36.0 | 2.40 | 54.0 | 3.60 | 5.7 |
| | | 110 | 559 | 9.4 | 0.63 | 1036 | 41.6 | 2.77 | 47.8 | 3.19 | 5.1 |
| | Avg. | 113 | 537 | 9.4 | 0.63 | 1060 | 38.8 | 2.59 | 50.9 | 3.39 | 5.4 |
| CW-F2 | + | 96 | 559 | 8.0 | 0.53 | 771 | 21.3 | 1.42 | 30.2 | 2.01 | 3.8 |
| | * | 121 | 526 | 8.2 | 0.55 | 997 | 29.8 | 1.99 | 38.0 | 2.53 | 4.6 |
| | Avg. | 109 | 543 | 8.1 | 0.54 | 884 | 25.6 | 1.71 | 34.1 | 2.27 | 4.2 |
| CW-C1 | + | 157 | 584 | 7.5 | 0.50 | 1172 | 34.5 | 2.30 | 49.6 | 3.31 | 6.6 |
| | | 180 | 376 | 6.1 | 0.40 | 1089 | 41.5 | 2.77 | 51.3 | 3.42 | 8.4 |
| | Avg. | 169 | 480 | 6.8 | 0.45 | 1131 | 38.0 | 2.55 | 50.5 | 3.37 | 7.5 |
| CW-C2 | + | 124 | 485 | 9.3 | 0.62 | 1155 | 40.7 | 2.71 | 48.1 | 3.21 | 5.2 |
| | | 139 | 397 | 7.3 | 0.49 | 1008 | 32.5 | 2.17 | 55.6 | 3.71 | 7.6 |
| | Avg. | 132 | 441 | 8.3 | 0.55 | 1082 | 36.6 | 2.44 | 51.9 | 3.46 | 6.4 |

Table 2: characteristics loading responses for models

Four 1/3 scale DSC wall specimens, varying with face plate profile and tie stud spacing. Table 2 shows the various responses of the wall. The corrugated DSC wall with standard bolt spacing were able to achieve an ultimate drift ratio around 3.4% and ductility ratio greater than 5.4. steel tube fracture, steel faceplate buckling and concrete crushing at wall bottom failure in flexure for all specimen was observed. Corrugated DSC wall with double tie bolt spacing had higher initial stiffness, ductility ratio and energy dissipation compared with flat DSC wall. On average less than 6% difference was obtained between predicted and measured capacity of lateral strength or drift.

Khandaker M.A. Hossain, Shahryar Rafiei, Mohamed Lachemi, Kamran Behdinan[4] (2015) investigated the two skinned profile steel sheeting and an infill concrete composite wall system behavior under inplane cyclic shear loading. DSCW specimen with overall height 1626mm high by 720 mm wide. To generate the composite action between steel-seet-concrete, the intermediate fasteness along the height and width of wall were provided. The type of loads acting on the wall is shown in table 3. To two types of concrete used namely self-consolidating concrete (SCC) and highly ductile engineered cementitous composite concrete (ECC) with cold formed profile steel sheet with same geometry and two different yield strength were incorporated. Figure 5 showed the casting of DSCW specimen.

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Table 3: test matrix, dimensions and material property.

| Wall designation | Loading | Concrete properties (mean values) MPa | | | Steel properties (mean values) stress and modulus of elasticity in MPa | | | | | |
|------------------|-----------|--|-----|--------|---|---------|----------|--------|---------|--|
| | | fc | f'ı | Ec | Yield | | Ultimate | | Es | |
| | | | | | Stress | Strain | Stress | Strain | | |
| DSCW1-SCC-HSS | Monotonic | 48.0 | 3.9 | 25,100 | 552 | 0.00272 | 577 | 0.01 | 202,940 | |
| DSCW3-ECC-HSS | Monotonic | 40.2 | 5.4 | 24,500 | 552 | 0.00272 | 577 | 0.01 | 202,940 | |
| DSCW5-SCC-MSS | Monotonic | 35.7 | 3.3 | 25,010 | 354** | 0.00171 | 420 | 0.10 | 206,980 | |
| DSCW2-SCC-HSS | Cyclic | 48.1 | 3.8 | 25,100 | 552 | 0.00272 | 577 | 0.01 | 202,940 | |
| DSCW4-ECC-HSS | Cyclic | 39.4 | 4.9 | 24,500 | 552 | 0.00272 | 577 | 0.01 | 202,940 | |
| DSCW6-SCC-MSS | Cyclic | 34.6 | 3.2 | 25,100 | 354 | 0.00171 | 420 | 0.10 | 206,980 | |

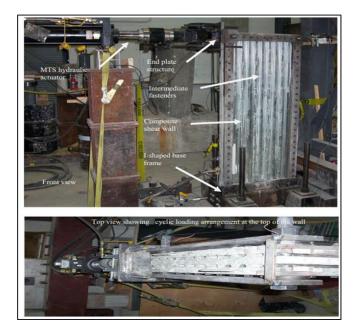


Fig. 5. Casting of DSCW specimens and typical DSCW specimen after casting.



Figure 6: Failure mode of steel sheet and concrete core under monotonic loading.

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Fig. 7. Failure mode of steel sheet and concrete core under cyclic loading.

The benefits were demonstrated through more ductile failure when mild steel over high-strength steel was used. The failure mode of steel sheet and concrete core under monotonic and cyclic loading is shown in figure 6 and 7 respectively. As compared to SCC wall the ECC wall showed lower stiffness degradation, higher displacement ductility and higher energy dissipation.

Xiaomeng Zhang, Ying Qin, Zhihua Chen, Li Jie [5] (2016) proposed an innovative T-shaped composite wall called as bundled lipped channel concrete (BLC-C). The flange of the proposed wall consist of cold formed square hollow section (SHS) at the middle and two lipped channel through complete joint penetration (CJP) welds. The wall web was made by connecting several lipped channels through complete joint penetration weld. The steel channels and tubes enclosed into compartment filled with concrete. Five specimen was subjected under constant axial load subjected to cyclic lateral load as shown in figure 8. The parameter studied in the experiments include the wall configuration, cross section of wall, steel sheet thickness, axial force ratio, and presence of shear stud.

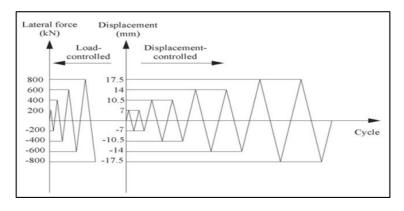


Figure 8. loading protocol

The specimen shows ductile manner and stable hysteresis behavior, local buckling was occurred at the base of the wall. The seismic behavior was evaluated as strength, stiffness, deformation, ductility and energy dissipation capacity as shown in table 4.



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Table 4: test results

| Specimen | Loading direction | Initial stiffness (kN/m) | Yield load P _y (kN) | Yield displacement d _y (mm) | Yield drift (rad) | Peak load P _m (kN) | Displacement at peak load d _m (mm) | Drift at peak load (rad) | Ultimate displacement d _u (mm) | Ultimate drift (rad) | Ductility µ | P _m /P _y |
|----------|----------------------|-----------------------------|-----------------------------------|---|----------------------|----------------------------------|---|--------------------------------|---|-------------------------|----------------|--------------------------------|
| BLC-C-T1 | (+) | 71,871 | 1297 | 20.8 | 0.0080 | 1452 | 28.1 | 0.0108 | 48.6 | 0.0187 | 2.34 | 1.12 |
| | (-) | 69,239 | 1338 | 23.2 | 0.0089 | 1457 | 31.9 | 0.0123 | - | - | - | 1.09 |
| BLC-C-T2 | (+) | 66,952 | 1269 | 22.4 | 0.0086 | 1412 | 29.9 | 0.0115 | 45.0 | 0.0173 | 2.01 | 1.11 |
| | (-) | 68,034 | 1364 | 25.5 | 0.0098 | 1528 | 36.5 | 0.0141 | 59.5 | 0.0229 | 2.33 | 1.12 |
| BLC-C-T3 | (+) | 73,273 | 1282 | 19.8 | 0.0076 | 1393 | 26.1 | 0.0100 | 44.6 | 0.0172 | 2.25 | 1.09 |
| | (-) | 73,839 | 1435 | 24.0 | 0.0092 | 1584 | 41.0 | 0.0158 | 70.0 | 0.0269 | 2.92 | 1.10 |
| BLC-C-T4 | (+) | 163,050 | 2641 | 18.1 | 0.0070 | 3043 | 40.4 | 0.0155 | 58.0 | 0.0223 | 3.20 | 1.15 |
| | (-) | 146,300 | - | - | - | - | - | - | - | - | - | - |
| BLC-C-T5 | (+) | 152,007 | 2754 | 21.6 | 0.0083 | 3090 | 49.1 | 0.0189 | 67.5 | 0.0260 | 3.12 | 1.14 |
| | (-) | 146,374 | - | - | - | - | - | - | - | - | - | - |

III.CONCLUSION

In summary, researchers studied the behavior of composite shear wall of different shapes like L-shaped, C-shaped, T-shaped, flat and corrugated shear wall under cyclic loading is done. The research gap identified and the concluding remark highlights the need for further research to improve the design and construction of these structure and develop more accurate design guidelines.

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